

GEOTECHNICAL INVESTIGATION REPORT HIGH SIERRA SUBDIVISION 129 LOTS IN 14th, 15th & 16th FILINGS BILLINGS, MONTANA

PREPARED FOR:

Mr. Landy Leep High Sierra II, Inc. 175 North 27th Street, Suite 940 Billings, MT 59101



January 22, 2020

Mr. Landy Leep High Sierra II, Inc. 175 North 27th Street, Suite 940 Billings, MT 59101

SUBJECT:

Geotechnical Investigation Report

High Sierra Subdivision, 14th, 15th, & 16th Filings

Billings, Montana

Dear Mr. Leep:

This report presents the results of our geotechnical investigation for the High Sierra Subdivision Project. The site location and boring locations are shown on the Vicinity and Site Maps shown on Plate 1 at the end of this report. The projects consists of 129 residential lots in the 14th, 15th & 16th Fillings.

Our recommendations contained in this report are based on exploratory borings, laboratory testing, engineering analysis and preparation of this report. The recommendations required to design residential foundations and complete utility installation are contained in the attached report. These conclusions and recommendations, along with restrictions and limitations on these conclusions, are discussed in the attached report.

We appreciate this opportunity to be of service to you, and look forward to future endeavors. If you have any questions regarding this report or need additional information or services, please feel free to call the undersigned.

Sincerely,

RAWHIDE ENGINEERING, INC.

Jason A. Frank

Principal

Enclosures:

Report (1 hard copy, 1 pdf)

1-22-20



6871 King Avenue West, Suite G1K, Billings, MT 59106 (406) 969-5305

October 26, 2020

Mr. Landy Leep High Sierra II, Inc. 175 North 27th Street, Suite 940 Billings, MT 59101

SUBJECT:

Addendum to the Geotechnical Investigation Report

High Sierra Subdivision, 14th, 15th & 16th Filings

Billings, Montana

Dear Mr. Leep:

This letter is an addendum to the above geotechnical investigation. The report was completed January 22, 2020 and at that time was based on a preliminary plat. The final plat has some lots that were moved and renamed. There are 125 total lots instead of the 129 listed in the report. The following table lists the correct lots that this report applies to.

LOTS INVESTIGATED FOR THIS REPORT

Filing, Block No.	Lots (No. of Lots)
14th Filing – Block 21	1-2 (2)
14th Filing - Block 23	1-2, 29-38 (12)
14th Filing - Block 26	1 (1)
14th Filing - Block 27	1-25 (25)
14th Filing – Block 28	17-24 (8)
14th Filing - Block 31	1-16 (16)
14th Filing – Block 32	1-18 (18)
14th Filing – Block 33	1-6 (6)
15th Filing - Block 26	1-9 (9)
15th Filing – Block 28	5A-16 (12)

15th Filing – Block 29	29-30 (2)
15th Filing – Block 30	1-4 (4)
16th Filing – Block 31	17-25 (9)
16th Filing – Block 32	19 (1)

We appreciate this opportunity to be of service to you, and look forward to future endeavors. The new plat is attached.

Sincerely,

RAWHIDE ENGINEERING, INC.

Robert W Yukis

Robert W. Kukes, P.E.

Principal

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GEOTECHNICAL INVESTIGATION REPORT HIGH SIERRA SUBDIVISION 129 LOTS IN 14th, 15th & 16th FILINGS BILLINGS, MONTANA

INTRODUCTION

Project Description

This residential subdivision development consists of 129 residential lots located in the High Sierra Subdivision in Billings, Montana. This project consists of the 129 Lots in High Sierra in the 14th, 15th & 16th Filings shown on the site map and in the following table. The vicinity and site maps are shown on Plate 1 following this report. The site is currently agricultural land covered with vegetation.

LOTS INVESTIGATED FOR THIS REPORT

Filing, Block No.	Lots (No. of Lots)
14th Filing – Block 21	1-2 (2)
14th Filing – Block 23	1-2, 29-38 (12)
14th Filing – Block 26	1-6, 10 (7)
14th Filing – Block 27	1-25 (25)
14th Filing – Block 28	17-24 (8)
14th Filing – Block 31	1-16 (16)
14th Filing – Block 32	1-18 (18)
15th Filing – Block 26	1-9 (9)
15th Filing – Block 28	5A-16 (12)
15th Filing – Block 29	29-30 (2)
15th Filing – Block 30	1-4 (4)
16th Filing – Block 31	17A-29 (13)
16th Filing – Block 32	8 (1)

Scope of Services

Our scope of services for this project consisted of the following:

- 1. Drilling 18 exploratory borings to depths of 15 to 25 feet below existing site grades.
- 2. Laboratory testing to determine the characteristics of the site soils for use in engineering design.
- 3. Engineering analysis to aid in the design of structure foundations and utility installation.
- 4. Provide information as to the existing groundwater conditions at the time of our exploration.
- 5. Provide recommendations for earthwork and construction on the site.

This study did not include evaluations of site seismicity, liquefaction, faulting, or other potential geologic or environmental hazards. This study did not include a groundwater study or the design of a dewatering system.

Authorization

Authorization to proceed with our work on this project was provided on December 16, 2019.

Professional Statements and Limitations

Recommendations presented in this report are governed by the physical properties of the soils encountered in the exploratory borings, laboratory testing, current groundwater conditions, the project layout and design data described in the following proposed construction section.

The recommendations presented in this report are based on exploratory boring locations shown on the site map. Variations in soils may exist between the explored locations and the nature and extent of soil variations may not be evident until construction occurs. If subsurface conditions other than those described in this report are encountered and if project design and layout is substantially altered from the information in this report, Rawhide Engineering should be notified so that recommendations can be reviewed and amended, if necessary.

This report has been prepared for design purposes for our client and specifically for this project in accordance with the generally accepted standards of practice at the time the report was written. No warranty, either expressed or implied, are intended or made.

Other standards or documents referenced in any given standard cited in this report, or otherwise relied upon by the authors of this report, are only mentioned in the given standard; they are not incorporated into it or "included by reference," as that latter term is used relative to contracts or other matters of law.

PROPOSED CONSTRUCTION

It is our understanding that this project will include the construction of 129 residential structures which are anticipated to be one to two story wood framed structures with concrete basements, crawl spaces or conventional stemwall foundations. Exact information on each structure was not available at the time of this report. Rawhide Engineering has estimated that the structural loads for these structures will have continuous footings loads of 2 to 3 kips per lineal foot for long term loading conditions based on experience with similar projects. The project will also include installation of utilities and interior streets.

FIELD INVESTIGATION

In order to determine and evaluate the subsurface conditions across the site, 18 exploratory borings were completed using a truck mounted drill rig equipped with hollow stem and solid stem augers. Boring depths were 15 to 25 feet below the existing ground surface. The location of the borings shown on the Site Map were dimensioned from property corners with the site map provided. This location should be considered accurate only to the degree implied by the method used.

The field investigation was under the direct control of an experienced member of our geotechnical staff who logged the soil conditions for each boring. Samples were obtained from driving a 2-inch Standard Penetration Sampler 18 inches using a 140 pound hammer falling 30 inches. The blow counts recorded on the boring logs were determined by counting the number of blows for the last 12 inches of the drive sample. Bulk auger cuttings were also obtained for further testing. The SPT and bulk samples were examined by field personnel, logged and sealed to prevent moisture loss prior to laboratory testing. After completion, the groundwater level in the boring was recorded and the borings were backfilled using drill cuttings.

The boring logs included at the end of this report are labelled B-1 through B-18. A boring log legend and a description of the Unified Soil Classification System used to identify the soils is included with the boring logs.

LABORATORY TESTING

A laboratory testing program was utilized to provide the necessary data for engineering analysis of this project. The testing was used to evaluate the index and engineering properties specifically for the conditions encountered during our field exploration. The following program was used for this project.

Moisture Content Tests - ASTM D2216

Moisture content tests were conducted on selected samples obtained from the site. These tests were used to aid in identifying the current soil conditions and aid in classifying the soils. Moisture content tests are shown on the boring logs.

Soil Classification Tests - ASTM D422, D1140, D4318, D2487 and D2488

In order to classify the soils according to the Unified Classification System, soil gradations and Atterberg Limits test were conducted on selected samples. The results of this testing is shown below and on the boring logs.

Gradations and Atterberg Limits Tests

Percent Passing									
Sieve Size	B-3 @ 6.0 – 8.0'	B-9 @ 4.5 – 6.0'	B-13@ 4.5-6.0'						
3/8	100	100	100						
No. 4	99	100	99						
No. 10	91	98	92						
No. 20	84	94	82						
No. 40	77	90	74						
No. 80	67	85	62						
No. 200	59	78	50						
Plastic Index	9.6	12.4	6.2						
Unified Classification	Sandy Lean Clay (CL)	Lean Clay with Sand (CL)	Sandy Silty Clay (CL-ML)						

SITE CONDITIONS

The site is located east and north of the existing High Sierra Subdivision. The site is currently agricultural grazing land. The site generally slopes to the east and north, but has some significant elevation changes. The site has two drainages that run through the property to the east and north. Both had water in them at the time of our field exploration.

The site is bordered by residential development on the south and east and agricultural grass land on the remaining sides. Drainage on the site consists of infiltration and runoff to the northeast.

SUBSURFACE SOILS AND GROUNDWATER

The soil conditions encountered on the site generally consist of a layer of vegetated topsoil with organics extending to a depth of 0.5 feet below the existing surface. Beneath the topsoil we encountered layers of sandy silty clay and sandy lean clay. The depths of these layers varied widely across the site. These fine grained soils were medium stiff to soft with a low to moderate plastic index. Beneath the upper fine grained soils we encountered highly weathered sandy shale to the depths explored of 15 to 25 feet. The shale bedrock becomes more competent with depth. These filings have sandstone caprock scattered throughout much of the site and it will have to be removed during construction of the interior streets and to excavate foundations. This sandstone is hard and may require large equipment to remove. The highly weathered sandy shale is lean clay with sand when mechanically broken down. The sandy shale has a low to moderate swell potential. Groundwater was encountered in three of the borings which are adjacent to the existing drainages during our exploration in January 2020. The boring logs should be reviewed carefully to determine the depth of each soil strata at the different areas of the site.

RECOMMENDATIONS

Prior to construction, surface soils should be removed from the site or stockpiled for use in non-structural areas. It appears about 0.5 feet can be used as a reasonable estimate for average depth of stripping. All undocumented fills, trash, vegetation and all abandoned underground utilities, wells, and foundations should be removed from the site if they are present. Excavations resulting from removal operations should be cleaned of all loose material and widened as necessary to permit access to compaction equipment.

Due to the changes in elevation across the site, cuts and fill may be required to construct the interior streets and building pads. All fills greater than 4 feet should be compacted to 98% of ASTM D698 to reduce potential settlement.

Excavations

The contractor is ultimately responsible for the safety of workers and should strictly observe federal and local OSHA requirements for excavation shoring and safety. All temporary slopes should comply with OSHA requirements for Type A soils in the upper soils and Type B in the lower shale and sandstone layers During wet weather, runoff water should be prevented from entering excavations.

It appears that excavation for footings and utility trenches can be readily made with either a conventional backhoe or excavator in the native soil materials. We expect the walls of the footing trenches in the near surface fine grained soils or weathered bedrock to stand near vertically without significant sloughing. If trenches are extended deeper than five feet or are

allowed to dry out, the excavations may become unstable and should be evaluated to verify their stability prior to occupation by construction personnel. Shoring or sloping of any deep trench walls may be necessary to protect personnel and provide temporary stability. All excavations should comply with current OSHA safety requirements for Type A or Type B soils. (Federal Register 29 CFR, Part 1926).

We recommend all backfill be compacted to a minimum compaction of 97% of the maximum dry density as determined by ASTM D698. The moisture content of compacted backfill soils should be within 2% of the optimum. Poor compaction in utility trench backfill may cause excessive settlements resulting in damage to the pavement structural section or other overlying improvements. Compaction of trench backfill outside of improvement areas should be a minimum of 90% relative compaction.

Material - Pipe bedding shall be defined as all material within six inches of the perimeter of the pipe. Backfill shall be classified as all material within the remainder of the trench. Material for use as bedding shall consist of clean, granular materials, and shall conform to requirements for bedding material listed in Section 02221 of the Standard Specifications.

Placement and Compaction - Pipe bedding shall be placed in thin layers not exceeding eight inches in loose thickness, and conditioned to the proper moisture content for compaction.

All other trench backfill shall be placed in thin layers not exceeding eight inches in loose thickness, conditioned to the proper moisture content, and compacted as required for adjacent fill. If not specified, backfill should be compacted to at least 97% relative compaction in areas under structures, utilities, roadways, parking areas, concrete flatwork, and to 90% relative compaction in undeveloped areas.

Foundations

This site varies widely across the total acreage. Some of the foundations will be located on weathered shale bedrock. Portions of the site will have foundations in the medium stiff to soft sandy lean clay soils. We are providing two alternative foundation recommendations for this project.

Shallow foundations in areas which encounter the weather shale bedrock should have the entire building envelope over excavated 1.5 feet in depth and extend laterally 1.5 feet beyond the edge of the footing. The over excavated material should be replaced with compacted structural fill. Utilizing the structural loads estimated for residential construction, and an allowable bearing pressure of 2,500 pounds per square for compacted structural fill, a settlement of less than ½ inch was estimated.

In areas where we encountered soft sandy lean clay to depths below expected foundation elevations, we are recommending the use of helical piers or grouted micro piles into the shale bedrock to reduce the potential for settlement. Depending if the structure has a basement, crawl space or conventional stem walls, the piers would need to be approximately 5 to 10 feet in length to reach 3 to 5 feet into the shale bedrock. These two pier alternatives would have capacities of 25 for 40 kips depending on the system used and the depth of the pier. Pier spacing should be designed by the structural engineer.

Structural fill under foundations shall be placed in layers, moisture conditioned, and compacted to 98% of ASTM D698. Exterior continuous foundations should be embedded a minimum of 3.5 feet below lowest adjacent exterior finish grade for frost protection and confinement. Interior footings should be bottomed at least 12 inches below lowest adjacent finish grade for confinement. Wall foundation dimensions should satisfy the requirements listed in the latest edition of the International Residential Code. Reinforcing steel requirements for foundations should be provided by the design engineer.

The allowable bearing pressures, indicated above, are net values, therefore, the weight of the foundation and backfill may be neglected when computing dead loads. Allowable bearing pressures may be increased by one-third for short-term loading such as wind or seismic. Resistance to lateral loads in the upper sandy, silty clay or sandy lean clay soils may be calculated using an allowable passive equivalent fluid unit weight of 210 pounds per cubic foot and an allowable coefficient of friction of 0.37 applied to vertical dead loads. Both passive and frictional resistances may be assumed to act concurrently. An allowable active equivalent fluid pressure of 38 pounds per cubic foot may be used.

The International Building Code (IBC) site class for this project is Class C.

Structural Fill

Structural fill will be used beneath the entire building envelope and should consist of dense gravel with sand and conforming to the following gradation and plastic index.

Sieve Size	Percent Passing
3 Inch	100%
No. 4	25-65%
No. 200	<20%
Plastic Index	12 or less

All structural fill shall be placed in eight inch loose lifts and uniformly moisture conditioned to within +/-2% of optimum moisture content. The contractor shall provide and use sufficient equipment of a type and weight suitable for the conditions encountered in the field. The

equipment shall be capable of obtaining the required compaction in all areas, including those that are inaccessible to ordinary rolling equipment.

Compaction Requirements

COMPACTION REQUIREMENTS							
Structural Fill Beneath Foundations	98% of ASTM D698						
Backfill Against Foundations	95% of ASTM D698						
Utility Trench Backfill	97% of ASTM D698						

The following table lists the compaction requirements for structural fill, foundation backfill, utility trench backfill and street subgrade preparation.

Concrete Slab-on-Grade Construction

Prior to constructing concrete slabs, the upper six inches of slab subgrade should be scarified, moisture conditioned to within 2% of optimum, and uniformly compacted to at least 95% of maximum dry density as determined by ASTM D698. Scarification and compaction will not be required if floor slabs are to be placed directly on undisturbed compacted structural fill.

All concrete floor slabs should have a <u>minimum</u> thickness of four inches. Slab thickness and structural reinforcing requirements within the slab should be determined by the design engineer. At least four inches of crushed base aggregate should be placed beneath slab-on-grade floors to provide uniform support. The aggregate base should be compacted to a minimum of 95% relative compaction.

We recommend that the base course be placed within three to five days (depending on the time of year) after moisture conditioning and compaction of the subgrade soil. The subgrade should be protected against drying until the concrete slab is placed.

In floor slab areas where moisture sensitive floor coverings are planned, an impermeable membrane (e.g. 10-mil thick polyethylene) should be placed over the base course to reduce the migration of moisture vapor through the concrete slabs. The impermeable membrane should be installed as required by the flooring manufacturer. Current literature from the American Concrete Institute and the Portland Cement Association recommend that the impermeable membrane is placed on top of the base course directly below the concrete slab.

Site Drainage and Infiltration

Final elevations at the site should be planned so that drainage is directed away from all foundations and concrete slabs. Parking areas should be designed to drain surface water off the sight and away from structures. In accordance with the International Residential Code, downspouts with 6 foot extensions should be used. Positive drainage away from all foundations should have 6 inches of fall in the first 10 feet away from the foundations. If sufficient room is not available to construct the 10 foot slope, drainage swales should be constructed as far from the foundations as possible. Plants and landscaping should not be allowed within 3 feet of foundations. Landscaping features such as curbing should not be allowed to create ponding adjacent to the foundations.

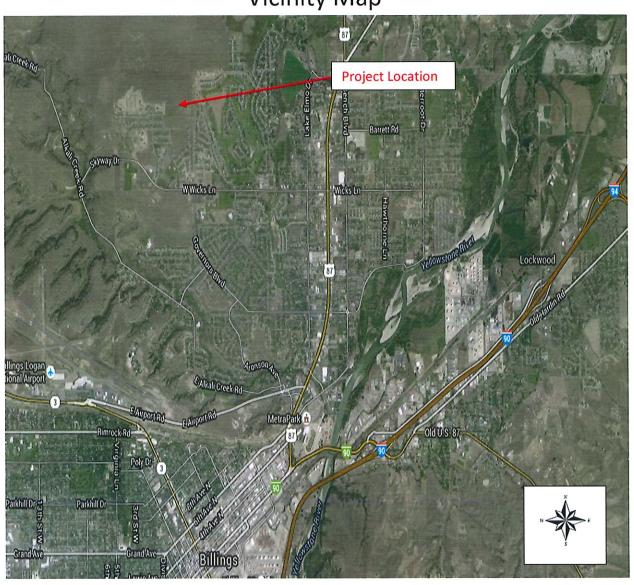
A 25 foot boring was completed in the northeast portion of the site for a detention pond. This pond is outside of the limits of these filings and consisted of a 3.5 foot layer of sandy lean clay and then weathered sandy shale to the depths explored of 25 feet below existing site grades. The weathered shale will have minimal infiltration as ground water is typically perched on the shale. Infiltration rates would be as low as 0.01 inches per hour.

Failure to follow all of the recommendations in this report including grading, foundation design and especially drainage and landscaping will render this report null and void.

APPENDIX A

Plates

Vicinity Map





PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	LABORATORY TESTING									STING
Depth (ft)		/ 6 in.		USCS Symbol	BORING NUMBER: 1	Consistency	Water Content (%)	u	Minus #200 (%)	Sample Recovery
Dept	Sample Type	Blows / 6 in.	Soil Pattern	NSCS	MATERIAL DESCRIPTION AND COMMENTS	Cons	Water (Plastic (F	Minus (%	San
1 =				CL-ML	Topsoil with Vegetation Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
2 - 3 -				CEM	Completely Weathered Sandstone - Brown/Tan/Red, Moist, Medium Dense/Dense, Granular Non-Plastic					
4 - 5 - 6 -		14 15 17		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.2
7 8 9										
10		18 18 18				S				1.0
12										
14 ·										
16					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
17 18										
19										
120	7									

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

RawhideCLIENT:High Sierra IIEngineering Inc.LOCATION:Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	SAMPLES _ LABORATORY TESTING									
	$\overline{}$	MPL	ES	loo		y				STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 2 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 =				CL-ML	Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 =				CL	Sandy Lean Clay - Brown, Moist, Medium Stiff to Soft, Moderate Plastic Index					
5 -		6 4 4				F				1.3
7 -										
8 -										
9 =										
10 -		16 18 20		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	s				0.8
12 -										
14 =					*					
15 =					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
17 =										
18 -										
20 -								,		

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	SAMPLES LABORATORY TESTING									OTINIC
	SA	MPL	ES	10		<u>_</u>				
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 3 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 -				CL-ML	Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 =	•			CL	Sandy Lean Clay - Brown, Moist to Wet, Medium Stiff to Soft, Moderate Plastic Index					
4 - 5 -	1	5 5				F	25.4	9.6	59.4	1.5
6 -		5								
) ⁷ • 8 •			\leq	-	Perched Groundwater Level at 7.9 Feet					
9 • 10 •		17 15 19		PCEM	Highly Weathered Sandy Shale - Dark Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.0
12										
13										
15 ·					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Encountered at 7.9 Feet					
17	-									
18										
20	-		۵							

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	Engineering Inc.									
	SA	MPL	ES	ol				RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 4 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 = 2 = 3 =				CL-ML	Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
4 - 5 - 6 -		8 6 6		CL	Sandy Lean Clay - Brown, Moist, Medium Stiff, Moderate Plastic Index	F				1.2
7 -										
9 - 10 - 11 -		18 18 19		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.2
12 • 13 •										
15 ·					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
17										
18 ·										
20										

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings
High Sierra II

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

_	Engineering Inc. LOCATION									
	SA	MPL	ES	-			ABOF	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 5 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 -				CL-ML	Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 =				CL	Sandy Lean Clay - Brown, Moist to Wet, Medium Stiff, Moderate Plastic Index					
5 -	1	7 5 4				F				1.0
7 - 8 -			\neq	<u></u>	Perched Groundwater Level at 8.2 Feet	ä				
9 = 10 = 11 =		15 15 15		PCEM	Highly Weathered Sandy Shale - Dark Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				0.8
12 -										
14 -					Desire Fords at Approximately 45 O Foot Double					
16 -	$\frac{1}{2}$				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Encountered at 8.2 Feet					
17 -										
19 -	1									
-		-				100				

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes
DATE: 1/3/20

ELEVATION:

				gineerii						
	SA	MPL	ES	lo		1	LABOR	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 6 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 - 2 - 3 -				CL-ML	Sandy Silty Clay - Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
4 -		8		CL	Sandy Lean Clay - Brown, Moist, Medium Stiff, Moderate Plastic Index					
5 - 6 - 7 -	\	12 12		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) -	F				1.3
8 - 9 -					Low/Medium Plastic Index					
10 =	1	18 20 19				s				0.9
12 -										
14 -										
16 -	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
18 -	$\frac{1}{2}$									
20										

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings High Sierra II

Engineering Inc. LOCATION: Billings, Montana

CLIENT:

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

				gineerii						-
	SA	MPL	ES	ol		_		RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 7 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 =				CL	Sandy Lean Clay - Brown, Moist to Wet, Medium Stiff to Soft, Moderate Plastic Index					
4 -					-	19				
5 -		5 4 5				F				1.4
7 -										
8 -	1									
9 -		2		, _	Perched Groundwater Level at 9.6 Feet					
10 -	1	2 3				So				0.7
11=		9		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index		**			
12 -					Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand					29
14 -										
15 -					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Encountered at 9.6 Feet					
17=				**						
18 -										
20 -										

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

				9	ig Inc.	S				
	SA	MPL	ES	lo				RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 8 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 = 2 = 3 = 4 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
5 =		7 7 8		CL	Sandy Lean Clay - Brown, Moist, Medium Stiff, Moderate Plastic Index	F				1.3
7 = 9 = 10 = 11 = 13 = 14 = 15 = 15		21 20 18		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				0.7
16 - 17 - 18 - 19 - 20 -					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	SA	MPL	ES					RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 9 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 - 2 - 3 -				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
4 –				DCEM	Highly Weathered Sandy Shale - Brown/Gray with White					
5 -		15 16 16		POLIVI	Motling, Dry/Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	s	15.3	12.4	78.1	1.5
\ _7_										
8 - 9 -										
10 -		25 27 26		PCEM	Weathered Sandy Shale - Brown/Tan/Gray, Moist, Very Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	VS				0.5
12 -									25.	
14 =										
15 =					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
17 -										
19=				0.5						
20 -										

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	0.4	BADI	- 777	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			AROF	ΑΤΟΙ	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern 6	USCS Symbol	BORING NUMBER: 10 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)		Sample Recovery
					Topsoil with Vegetation					
1 = 2 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 - 4 - 5 - 6 - 7 -		14 14 15		PCEM	Highly Weathered Sandy Shale - Brown/Gray with White Motling, Dry/Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.0
9 = 10 = 11 = 12 = 14 = 15		27 25 28		PCEM	Weathered Sandy Shale - Brown/Tan/Gray, Moist, Very Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	VS				0.5
15 • 16 • 17 • 18 • 19 • 20 •	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					

PROJECT: High Sierra Subdivision 14th, 15th, 16th Filings

CLIENT: High Sierra II Billings, Montana LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

1/3/20 DATE: **ELEVATION:**

		E,	Ras Eng	whide gineerin	ng Inc. LOCATION: Billings, Montana		TION:		13120	=
	SA	MPL	ES	_			LABO	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 11 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 -				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 - 4 - 5 - 6 -		13 14 16			Highly Weathered Sandy Shale - Brown/Gray with White Motling, Dry/Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.4
7 - 8 - 9 -		20								
10 = 11 = 12 = 13 = 14 = 14 = 15		22 24		PCEM	Weathered Sandy Shale - Brown/Tan/Gray, Moist, Very Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	VS				0.7
15 - 16 - 17 - 18 - 19 -					Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings High Sierra II

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	CA	MPLI	:e				LABOF	RATOR	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern 6	USCS Symbol	BORING NUMBER: 12 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	u I	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 - 4 - 5 -		11 14 15		PCEM	Highly Weathered Sandy Shale - Brown/Gray with White Motling, Dry/Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.2
7 = 8 = 9 = 10 = 11 = 12 = 13 = 14 = 14 = 14 = 15	$\left\{ \cdot \right\}$	25 28 26		PCEM	Weathered Sandy Shale - Brown/Tan/Gray, Moist, Very Stiff, Moderate Plastic Index Note: When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	VS				1.0
15 • 16 • 17 • 18 • 19 • 20	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

ELEVATION:

DRILLER: R. Kukes
DATE: 1/3/20

LABORATORY TESTING SAMPLES Symbol Consistency Water Content Plastic Index Depth (ft) Sample Type Blows / 6 in. Soil Pattern Minus #200 Recovery Sample **BORING NUMBER: 13** (PI) **USCS** MATERIAL DESCRIPTION AND COMMENTS Topsoil with Vegetation 1 Sandy Silty Clay - Light Brown/Brown, Moist, Medium Stiff, CL-ML Low/Moderate Plastic Index 3 4 5 F 17.4 6.2 1.5 50.2 6 Sandy Lean Clay - Brown/Dark Brown, Moist, CL Medium Stiff to Soft, Moderate Plastic Index 8 9 3 10 1.5 3 So 12 • PCEM Highly Weathered Sandy Shale - Brown/Gray/Green, Moist, Stiff, Moderate Plastic Index 13 = Note: When Mechanically Broken Down - Classifies as (CL) -Lean Clay with Sand 14 -15 = Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered 16 -17 = 18 -19 =

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

 Engineering Inc.
 LOCATION:
 Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

				zineerin			ADOL	ATO	OV TE	CTING
	SA	MPL	ES	lo		>				STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 14 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
3 - 4 - 5 - 6 - 7 -		10 13 15		PCEM	Highly Weathered Sandy Shale - Brown/Gray with White Motling, Dry/Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	S				1.2
8 = 9 = 10 = 11 = 12 = 13 = 14 •		24 22 24		PCEM	Weathered Sandy Shale - Brown/Dark Brown/Gray, Moist, Very Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand	VS				0.8
15 · 16 · 17 · 18 · 19 · 20	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

RawhideCLIENT:High Sierra IIEngineering Inc.LOCATION:Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

	C A	MPLI	=e				LABOR	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern 6	USCS Symbol	BORING NUMBER: 15 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	-	Sample Recovery
					Topsoil with Vegetation					
1 - 2 - 3 -				CL-ML	Sandy Silty Clay - Light Brown/Brown, Moist, Medium Stiff, Low/Moderate Plastic Index				rs.	
4 - 5 - 6 - 7 -		6 5 6		CL	Sandy Lean Clay - Brown/Dark Brown, Moist, Medium Stiff, Moderate Plastic Index	F				1.5
8 - 9 -		15		PCEM	Highly Weathered Sandy Shale - Brown/Gray, Moist, Stiff, Moderate Plastic Index Note:When Mechanically Broken Down - Classifies as (CL) -					
10 - 11 - 12 -	1	17 18			Lean Clay with Sand	S			.5	0.8
13 =	-									
15 - 16 - 17 - 18 -	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
19 -	$\left\{ \right.$									

PROJECT: High Sierra Subdivision
14th, 15th, 16th Filings

Rawhide CLIENT: High Sierra II
Engineering Inc. LOCATION: Billings, Montana

LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes
DATE: 1/3/20

ELEVATION:

			V/6	gineerii			ADO	ATO	OV TE	CTING
	-	MPL	ES	lo		>	LABOR			DNITE
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 16 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
					Topsoil with Vegetation					
1 = 2 = 3 =				CL-ML	Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index			S.		
4 - 5 -				PCEM	Highly Weathered Sandy Shale - Brown/Gray with White Motling, Dry/Moist, Stiff, Moderate Plastic Index					
6 -					Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand					
7 -										
8 -										
9 =	1									
10 -										
11 =				PCEM	Weathered Sandy Shale - Brown/Gray, Moist, Very Stiff, Moderate Plastic Index					
12 -	+				Note: When Mechanically Broken Down - Classifies as (CL) -					
13 =					Lean Clay with Sand					
14 =	1									
15 =	1				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
16 -					Groundwater was Not Encountered					
17 =										
18 -										
19										
20-										

PROJECT: High Sierra Subdivision 14th, 15th, 16th Filings

High Sierra II

CLIENT: Engineering Inc. LOCATION: Billings, Montana LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

R. Kukes DRILLER:

1/3/20 DATE: ELEVATION: __

	SA	MPL	ES	-				RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 17 MATERIAL DESCRIPTION AND COMMENTS	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
				a version and the	Topsoil with Vegetation					
1 - 2 - 3 -					Sandy Silty Clay - Brown/Light Brown, Moist, Medium Stiff, Low/Moderate Plastic Index					
4 -										
5 -				CL	Sandy Lean Clay - Brown, Moist, Medium Stiff					
7 -					Moderate Plastic Index					
8 -										
9 =				PCEM	Highly Weathered Sandy Shale - Brown/Gray with White Motling, Moist, Stiff, Moderate Plastic Index					
11 =					Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand					
12 -										
13=										
14 -					Paring Ende at Approximately 15 0 Foot Donth					
16 -	-				Boring Ends at Approximately 15.0 Feet Depth Groundwater Was Not Encountered					
17 =										
18 -										
≥0-										

Billings, Montana

PROJECT: High Sierra Subdivision 14th, 15th, 16th Filings High Sierra II **CLIENT:** Engineering Inc. LOCATION:

LOGGED BY: ____ J. Frank DRILL METHOD: Hollow Stem **DRILLER:** R. Kukes 1/3/20 DATE:

ELEVATION:

	SA	MPL	ES	_			LABOF	RATO	RY TE	STING
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 18	Consistency	Water Content (%)	Plastic Index (PI)	Minus #200 (%)	Sample Recovery
	Š	B	O ₂		MATERIAL DESCRIPTION AND COMMENTS		⊭	거		
					Topsoil with Vegetation					
1 - 2 - 3 -					Sandy Silty Clay - Brown with White Motling, Moist, Medium Stiff, Low/Moderate Plastic Index					
4 –										
5 =				CEM	Completely Weathered Sandstone - Light Brown/Tan/Red, Moist/Dry, Medium Dense/Dense, Granular Non-Plastic					
6 –										
7 -										
8 –				PCEM	Highly Weathered Sandy Shale - Brown/Dark Gray, Moist, Stiff,					
-					Moderate Plastic Index					
9 🗕					Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand					
					Lean Clay with Sand					
10 –										
11 =										
12 -										ē
40										
13=										
14 –										
15 =										
13.				DCEM	Weathered Sandy Shale - Brown/Dark Prown/Gray/Ped Maist					
16 –				PUEIN	Weathered Sandy Shale - Brown/Dark Brown/Gray/Red, Moist, Moderate Plastic Index					
17 =					Note:When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand					
18 –										
19 =										
20 -		-			Boring Log Continued Next Page				+-	
					Bonning Log Continuod Hoxer ago					

PROJECT: High Sierra Subdivision

14th, 15th, 16th Filings

 Rawhide
 CLIENT:
 High Sierra II

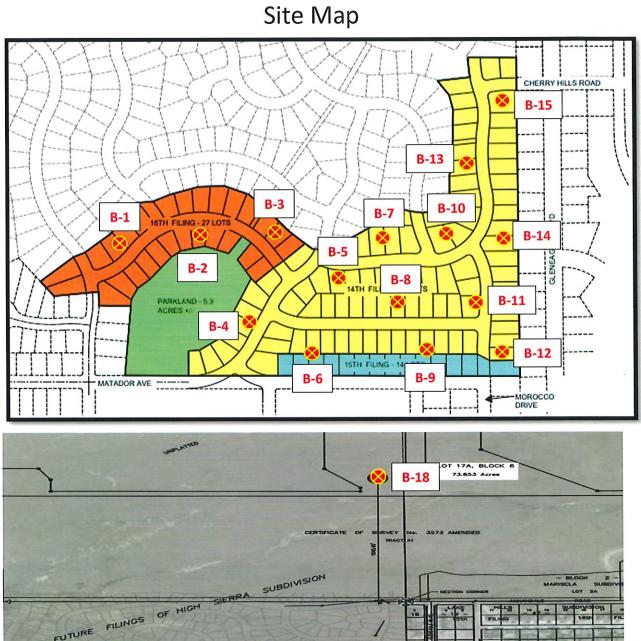
 Engineering Inc.
 LOCATION:
 Billings, Montana

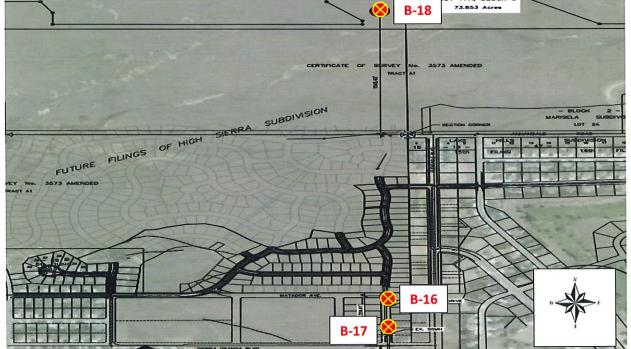
LOGGED BY: J. Frank

DRILL METHOD: Hollow Stem

DRILLER: R. Kukes

SAMPLES _ LABORATORY TESTING										
Depth (ft)	Sample Type	Blows / 6 in.	Soil Pattern	USCS Symbol	BORING NUMBER: 18 Continued MATERIAL DESCRIPTION AND COMMENTS	Consistency		Plastic Index (PI)		
21 = 22 = 23 = 24 = 25 = 26 = 27 = 30 = 31 = 32 = 34 = 35 = 36 = 37 = 38 = 39 = 40 = 40 = 40 = 40 = 40 = 40 = 40 = 4				PCEM	Weathered Sandy Shale - Brown/Dark Brown/Gray/Red, Moist, Moderate Plastic Index Note: When Mechanically Broken Down - Classifies as (CL) - Lean Clay with Sand Boring Ends at Approximately 25.0 Feet Depth Groundwater Was Not Encountered					
1										



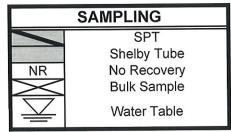




BORING LOG LEGEND

	MATERIAL	DESCRIPTION
Soil Pattern	USCS Symbol	USCS Classification
	FILL	Artificial Fill
	GP or GW	Poorly/Well graded GRAVEL
	GM	Silty GRAVEL
	GC	Clayey GRAVEL
	GP-GM	Poorly graded GRAVEL with Silt
	GP-GC	Poorly graded GRAVEL with Clay
	SP or SW	Poorly/Well graded SAND
	SM	Silty SAND
	SC	Clayey SAND
	SP-SM	Poorly graded SAND with Silt
	SP-SC	Poorly graded SAND with Clay
	SC-SM	Silty Clayey SAND
	ML	SILT
	MH	Elastic SILT
	CL-ML	Silty CLAY
	CL	Lean CLAY
	CH	Fat CLAY
	PCEM	PARTIALLY CEMENTED
	CEM	CEMENTED
	BDR	BEDROCK

		CONSISTENCY					
Col	ohesionless Soils Cohesive Soils Cementation		Cohesionless Soils		Cohesive Soils		Cementation
VL	Very Loose	So	Soft	МН	Moderately Hard		
L	Loose	F	Firm	Н	Hard		
MD	Medium Dense	S	Stiff	VΗ	Very Hard		
D	Dense	VS	Very Stiff				
VD	Very Dense						





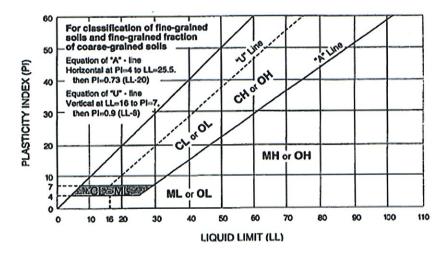
UNIFIED SOIL CLASSIFICATION SYSTEM

Criteria fo	for Assigning Group Symbols and Group Names Using Laboratory Tests*			Soli Classification		
				Group Symbol	Group Name®	
Coarse Grained Solls	Gravels	Cloan Gravels	Cu ≥ 4 and 1 ≤ Co ≤ 3 ^t	GW	Well-graded gravel	
More than 50% retained	More than 50% of coarse fraction retained on No. 4 sieve	Less than 5% fines ^c	Cu < 4 and/or 1 > Cc > 3t	GP	Poorly graded gravel*	
on No. 200 sieve		Gravels with Fines More than 12% fines ^c	Fines classify as ML or MH	GM	Silty graver's."	
011110. 200 51010			Fines classify as CL or CH	GC	Clayey gravel OH	
	Sands 50% or more of coarse fraction passes No. 4 slove	Clean Sands	Cu ≥ 6 and 1 ≤ Cc ≤ 36	sw	Well-graded sand	
		Less than 5% fines ^o	Cu < 6 and/or 1 > Cc > 3 st	SP	Poorly graded sand	
		Sands with Fines More than 12% fines ^o	Fines classify as ML or MH	SM	Silty sand ^{syu}	
			Fines Classify as CL or CH	sc	Clayey sand ^{ayu}	
Fine-Grained Soils	Sitts and Clays Liquid limit less than 50	inorganic	PI > 7 and plots on or above "A" line"	CL	Lean clay	
50% or more passes the			PI < 4 or plots below "A" line"	ML	Silveri	
No. 200 sieve		organic	Liquid limit - oven dried < 0.75	OL	Organic clay	
			Liquid limit - not dried	OL.	Organic silt ^{KLULO}	
	Silts and Clays Liquid firnit 50 or more	inorganic	PI plots on or above "A" line	СН	Fat clay****	
			PI plots below "A" line	мн	Elastic Sill****	
		organic	Liquid limit - oven dried < 0.75	ОН	Organic clay	
			Liquid limit - not dried	OH	Organic sitting	
Highly organic soils	Primar	ily organic matter, dark in	color, and organic odor	PT	Peat	

^{*}Based on the material passing the 3-in. (75-mm) sleve

$$E_{CU} = D_{60}/D_{10}$$
 $C_{C} = \frac{(D_{30})^2}{D_{10} \times D_{60}}$

PI plots below "A" line.





⁶ If field sample contained cobbles or boulders, or both, add "with cobbles or boulders, or both" to group name.

^CGravels with 5 to 12% fines require dual symbols: GW-GM well-graded gravel with silt, GW-GC well-graded gravel with clay, GP-GM poorly graded gravel with silt, GP-GC poorly graded gravel with clay.

DSands with 5 to 12% fines require dual symbols: SW-SM well-graded sand with slit, SW-SC well-graded sand with clay, SP-SM poorly graded sand with clay.

Fif soil contains ≥ 15% sand, add "with sand" to group name.

Gif fines classify as CL-ML, use dual symbol GC-GM, or SC-SM.

Hif fines are organic, add "with organic fines" to group name.

If soil contains ≥ 15% gravel, add "with gravel" to group name.

Jif Atterberg limits plot in shaded area, soil is a CL-ML, slity day.

Kif soil contains 15 to 29% plus No. 200, add "with send" or "with gravel," whichever is predominant.

^L if soil contains ≥ 30% plus No. 200 predominantly sand, add *sandy* to group name.

M If soil contains ≥ 30% plus No. 200, predominantly gravel, add "gravelly" to group name.

NPI ≥ 4 and plots on or above "A" line.

O PI < 4 or plots below "A" line.</p>

PPI plots on or above "A" line.